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<p>General</p> <p>1. These notes are to be read in conjunction with the written specifications and these drawings. In the event of conflict between the information on the drawings, these notes and the specifications, the more stringent requirements shall govern.</p> <p>2. The contractor is responsible for coordinating the Architectural, Mechanical and Electrical work with the work shown on these drawings. Discrepancies and/or interferences shall be reported to the architect immediately.</p> <p>3. Contractor shall verify all dimensions and existing conditions before beginning work. Check with electrical and mechanical contractors for conduits, pipe sleeves, etc., to be embedded in concrete, and masonry. Contractor shall take field measurements and be responsible for same.</p> <p>4. It is the contractor's responsibility to provide adequate shoring and bracing during construction to account for all forces, including but not limited to, forces from gravity, earth, wind, and unbalanced forces due to construction sequence.</p> <p>5. For conditions not expressly shown use details shown for obviously similar conditions.</p> <p>6. No openings shall be made in any structural member unless specifically shown on the structural drawings or unless approved in writing by the Structural Engineer.</p> <p>7. Contractor shall coordinate all support details for Architectural, Mechanical, Electrical and Plumbing equipments and provide any additional framing required subject to the approval of the Architect.</p> <p>8. Reproductions, in whole or in part, of Engineer's design documents, shall not be used as shop drawing plans and/or details.</p> <p>Design</p> <p>1. The design of the structure is in accordance with the Michigan Building Code, 2015 Edition.</p> <p>2. Warning: The structural integrity of the building shown on these plans is dependent upon completion according to plans and specifications. Structural members are not self-bracing and shall be shored and/or braced by the contractor as necessary until stabilized by virtue of completed connections.</p> <p>Loads</p> <p>1. Superimposed Dead Loads, allowed for in design: A. Typical = 15PSF</p> <p>2. Live Loads allowed for in design: A. Assembly Areas: a. Assembly: 100 PSF b. Lobbies: 100 PSF c. Light Storage: 125 PSF</p> <p>3. Wind Loads: A. Basic Wind Speed: 115 MPH B. Exposure Category: B C. Importance Factor: 1</p> <p>4. Snow Loads: A. Ground Snow Load (Pg): 25 PSF B. Exposure Factor (Ce): 1 C. Thermal Factor (Ct): 1 D. Importance Factor (I): 1 E. + Additional Drift Surcharge</p> <p>5. Earthquake Loads: A. Ss: 0.092g B. S1: 0.047g C. Seismic Design Category: B D. Seismic Site Class: D E. Importance Factor (I): 1</p> <p>Foundations</p> <p>1. Foundation construction shall be in accordance with the soil report by [G2 Consulting Group, 1666 Woodstee St., Troy, Michigan 48063, dated September 29, 2017, (G2 Project No. 173308).] The contractor shall obtain a copy of the soil report and become familiar with the requirements and recommendations therein.</p> <p>2. The slab on grade shall rest on a minimum of six (6) inches granular fill, compacted to at least 95% of the maximum density (as defined by the ASTM D1557 Modified Proctor Test.)</p> <p>3. All footings shall bear on undisturbed soil, having a minimum safe bearing capacity of [1500] psf. The Testing and Inspection Agency shall verify soil bearing capacity at each footing prior to placement of concrete. Notify architect of any variation from the anticipated bearing capacity for appropriate redesign or lowering of footing.</p> <p>4. Isolated footings shall be lowered or raised, and piers shall be added, reduced or increased in height as approved by the architect, where soil of the specified bearing capacity is found at a lower or higher elevation than shown on drawings.</p> <p>5. The bottoms of all exterior footings shall be minimum 3'-6" below finished grade. If the building will be under construction during freezing weather, all interior foundations shall be depressed 3'-6" below construction grade for frost protection. If such additional footing depth will cause undermining of adjacent existing footings or structures, provide shoring, bracing or underpinning as required or leave footing elevation as designed and provide continued protection and heat to prevent formation of frost below footing and adjacent to footing.</p> <p>6. Edges of footings shall not be placed at a greater than 1 vertical to 2 horizontal slope with respect to any adjacent footing or excavation, unless underpinning or shoring and bracing of existing footing or excavation is provided. Underpinning shall be done so as not to cause settlements of existing structure and shall be such that complete contact is achieved between new underpinning and existing concrete.</p> <p>7. Backfilling against basement walls shall not be done until the floor slabs at top and bottom of walls have been placed and have cured, or walls are properly shored and/or braced.</p> <p>8. Backfilling against foundation walls shall not be done until concrete has been cured to attain sufficient strength, 7 days minimum, and walls are properly shored and/or braced. Backfill foundation walls with earth on both sides of the wall by alternately placing backfill on each side so that height of backfill does not differ by more than 1'-0" from other side.</p> <p>9. All backfill within building lines shall be engineered granular fill placed under the full time supervision of a soil engineer and shall be compacted to achieve 95% Modified Proctor Density. Fill shall be placed in 9" maximum lifts.</p> <p>10. The contractor shall safeguard and protect all excavations, and adjacent structures, pavements, and utilities. All excavations shall be kept free of water. The contractor is responsible for the design, installation, maintenance, and removal of all shoring, bracing, and dewatering required to properly construct the foundations and to protect adjacent structures, pavements and utilities. Do not remove shoring such as sheet piling if it will cause settlement or damage to existing or new structures, pavement, and/or utilities.</p> <p>11. Maximum length of foundation wall placed in one operation shall not exceed 60 feet.</p> <p>12. The foundation contractor shall refer to Mechanical and Electrical Drawings for all locations of trenches, pits, conduits, etc. not shown on the Structural Drawings.</p>																																																																																																																																																																																																																																																														
<p>Masonry</p> <p>1. Concrete masonry units shall be normal weight units and shall conform to ASTM C-90 with a minimum design compressive unit strength of [1900] psi and a prism strength of [1500] psi. Concrete masonry construction shall conform to the following standards: A. "Building Code Requirements for Masonry Structures", ACI 530-08/ASCE 5-08 B. "Specifications for Masonry Structures", ACI 530.1-08/ASCE 6-08</p> <p>2. Brick masonry shall conform to the latest editions of the following Brick Industry Association Standards, in addition to the references in Note 1. A. "Building Code Requirements for Engineered Brick Masonry" B. "Recommended Practice for Engineered Brick Masonry" C. "Guide Specification for Brick Masonry"</p> <p>3. Brick units for exterior use as veneer shall conform to ASTM C-216 Grade S.W., with the following exceptions: A. The saturation coefficient (C/B) requirement shall not be waived as allowed in sections 5.1 and 5.2 unless sufficient engineering data and field records can be provided which demonstrates the units to be sufficiently durable and resistant to freeze-thaw deterioration. B. Initial rate of absorption (I.R.A.) shall not exceed 20 grams per minute per 30 square inches of face area. For cold weather construction of the bricks shall have a minimum I.R.A. of 6 grams per minute per 30 square inches of face area. Bricks shall not be wetted prior to placement.</p> <p>4. Brick manufacturer or distributor shall provide the Architect with brick test data and certificate of compliance with ASTM C-216 Grade S.W. and the above noted exceptions. Brick units shall not be shipped to the site until approval has been received from the Architect.</p> <p>5. Mortar for concrete and brick masonry shall conform to ASTM C-270, Type M for all below grade units. At above grade units, use Type S at concrete masonry units and Type N at clay masonry.</p> <p>6. Reinforcing for concrete masonry shall conform to ASTM A-615 Grade 60. Minimum bar lap shall be 24" or 48 bar diameters whichever is greater.</p> <p>7. Grout for bond beams and to fill cores of walls shall conform to ASTM C-476, with a minimum compressive cylinder strength of [3000] psi at 28 days. Grout shall be vibrated and re-vibrated after initial water loss to insure complete filling of cores.</p> <p>8. Place ladder type horizontal joint reinforcing with preformed lapped corner reinforcing at 16" c/c vertically in all masonry walls U.O.N. A. Joint reinforcing shall conform to ASTM A-951, be galvanized and have side wires of 9 gage minimum conforming to ASTM A-82 U.O.N. B. In brick and CMU cavity walls provide continuous ladder type horizontal joint reinforcing at 16" c/c vertical spacing with adjustable veneer tie system at 16" c/c horizontal spacing. The system shall permit movement of the tie both vertically and horizontally in the plane of the wall, but shall prevent movement normal to the wall such as #280 S.I.S. Dubi-Loop-Lok by Hohmann & Barnhard or #265 Lox-all-ladder type by Hohmann and Barnhard or engineer approved equal. C. At concrete walls with brick veneer provide continuous vertical dovetail slot anchors at 16" c/c horizontal spacing with triangular wire ties into the veneer at 16" c/c vertical spacing. D. All joint reinforcing and veneer ties shall be hot dip galvanized.</p> <p>9. Provide a continuous bond beam with 2-#5 continuous in the top course of all block walls, at locations where framing members are bolted to face of cmu walls, below beam bearings, and at locations indicated in the drawings. Bond beams shall have corner bars around corners and at wall intersections.</p> <p>10. Vertical control joints of a key type as indicated on the plans are to be placed in all masonry walls at intervals not to exceed 24 feet apart. The contractor shall furnish a control joint location plan for the Architect's approval if the plans do not sufficiently describe the location of all required control joints.</p> <p>11. The discontinuous ends of all masonry walls shall be solidly grouted a minimum of 8" or one block cell and reinforced for their full height with one #4 bar unless otherwise noted.</p> <p>Wood Framing</p> <p>1. All wood framing shall conform to the "Timber Construction Manual" by the American Institute of Timber Construction latest edition and the "National Design Specification for Wood Construction" by the American Forest & Paper Association latest edition.</p> <p>2. Plywood floor deck and wall sheathing shall be identified with the appropriate grade trademark of the American Plywood Association and shall meet the requirements of the latest edition of "U.S. Product Standards, PS 1 for Construction and Industrial Plywood". Plywood thickness is shown on the drawings.</p> <p>3. Glue for wood construction shall comply with specification AFG-01 of the American Plywood Association.</p> <p>4. Wood framing shall comply with the following minimum material specifications: A. Dimension Lumber Douglas Fir-Larch (DF) No. 2 or better (WVPA/NLGA) a. Bending Fb = 900 psi b. Shear Fv = 180 psi c. Mod. Elias. E = 1,600,000 psi B. Laminated Veneer Lumber Beams (LVL): a. Bending Fb = 2600 psi b. Shear Fv = 285 psi c. E = 1,900,000 psi C. Parallel Strand Lumber (PSL): a. Bending Fb = 2900 psi b. Shear Fv = 290 psi c. E = 2,000,000 psi 5. Roof Framing Specifications: A. Provide Hurricane Tie Downs (by Simpson or equiv.) @ roof framing members per manufacturer's specifications to bearing walls and beams below. B. Provide galvanized lightgage metal connectors such as joist, rafter and beam hangers, post bases and caps and tie-downs in accordance with manufacturer's specifications in all locations unless noted otherwise. C. All wood roof trusses shall be shop fabricated to the required dimensions and shall be capable of supporting all superimposed dead loads plus snow and/or live load as specified in the [2015] Edition of the [Michigan] Building Code. a. Shop drawings shall be submitted to the Architect for approval and shall bear the seal of a P.E. registered in the State of [Michigan]. b. The truss manufacturer is to engineer and provide all headers and girder trusses for flush framed truss support conditions. c. All trusses which flush frame into headers or girder trusses shall be connected with metal hangers designed by a [Michigan] P. E. and supplied by the truss manufacturer. Truss hangers are to include proper nails or bolts as required by their design. Pre-engineered truss hangers by "Simpson" will be acceptable. Size and capacity of truss hangers is to be shown on the shop drawings.</p>																																																																																																																																																																																																																																																														
<p>Structural Steel</p> <p>1. All structural steel work shall conform to A.I.S.C., "Specification for the Design, Fabrication and Erection of Structural Steel Buildings," 9th edition and the A.I.S.C., "Code of Standard Practice for Steel Buildings and Bridges", Latest Edition.</p> <p>2. Structural steel shall conform to the latest edition of the following ASTM designations: A. Structural steel shapes, except bars and plates: A-992 Grade 50 having a minimum yield strength of 50 ksi or A-572, Grade 50 having a minimum yield strength of 50 ksi unless noted otherwise on the plans. B. Steel bars and plates shall be ASTM A-572 Grade 50. (Fy=50 ksi). C. Hollow Structural Sections (Rectangular, Square and Round) - A500, Grade B having a minimum yield strength of 46 ksi. D. Steel Pipe - A-501 having a minimum yield strength of 36 ksi or A-53 Grade B type E or S having a minimum yield strength of 35 ksi.</p> <p>3. Bolts shall conform to the following ASTM designation, latest edition: High strength bolts - A325; anchor rods - F1554, Grade 55 unless noted otherwise on the drawings. All bolts shall be snug tight unless noted slip critical or S. C. on plans unless otherwise noted. All bolts that carry loads in tension shall be fully pretensioned.</p> <p>4. All bolts shall be 3/4" diameter, open holes 13/16" diameter, unless otherwise shown or noted. Use high strength bolts for steel framing connections.</p> <p>5. All welding electrodes shall conform to the E-70 series of the specification for mild steel arc welding electrodes ASTM A233, latest edition.</p> <p>6. All welding shall be done by certified, licensed welders and shall be in conformance with the structural welding code of the American Welding Society ANSI/AWS D1.1-latest edition.</p> <p>7. Deformed bar anchors shall be D2L deformed bar anchors, by Nelson Division of T.R.W. or equivalent anchors by Cox Industries, field welded to the structural steel by automatic welding process, not stick welded.</p> <p>8. Headed stud shear connectors shall be of diameter and length called out on drawing, and shall conform to ASTM A-108, Grade 1010 through 1020 and to the requirements of Section 7 of AWS D1.1, Type B. Weld to steel by automatic welding process (not stick welded).</p> <p>9. No penetrations are permitted through structural steel members unless indicated on structural drawings or approved by Engineer.</p> <p>10. Milled or butt welded stiffeners shall be provided on girders supporting a column and over all columns.</p> <p>11. Approval of the engineer shall be mandatory for the use of cutting torch in the field.</p> <p>12. All grout under steel plates shall be non-shrink "pre-mix" type and shall have a minimum compressive strength of 5,000 psi, tested in accordance with concrete specifications. Use non-staining grout at exposed locations.</p> <p>13. For miscellaneous steel construction not shown on Structural drawings, see Architectural, Mechanical and Electrical drawings.</p> <p>14. The steel fabricator may substitute heavier sections in place of the sections shown on the drawings to achieve economy of repetition, for availability or to take advantage of rolling mill production schedules so long as the changes are made known to the architect and engineer and are acceptable to both.</p> <p>15. All structural steel shall be painted with one shop-applied coat of rust inhibiting primer after surface preparation by SSPC 3 "power tool cleaning", unless noted otherwise. Do not paint portions of steel members that are to receive spray-on fireproofing, nor surfaces to receive welded shear studs. Steel structure that is permanently exposed on the exterior shall be hot dip galvanized according to ASTM A123.</p> <p>16. All structural steel exposed to view in the finished work including beams, columns, girts, trusses, bridging, bracing, and connections shall be classified as "Architecturally Exposed Structural Steel" (AESS) and shall meet the additional requirement given in Section 10 of the Code of Standard Practice for Steel Buildings and Bridges, 2000 Edition by AISC. See Architectural drawings and specs for locations and finish requirements of AESS.</p> <p>17. Reproductions, in whole or in part, of Engineer's design documents, shall not be used as shop drawing plans and/or details.</p> <p>Field Drilled Adhesive Anchors</p> <p>1. Field drilled adhesive bolts shall be Hilti HVA or H.I.T. System Adhesive Anchors as manufactured by HILTI Fastening Systems, Inc., or ITW Ramset/Redhead, Powers Fasteners, or Simpson Strong-Tie Anchor Systems equivalents.</p> <p>2. Anchors shall be installed per contract documents and per manufacturer's recommendations in coordination with special testing by an independent testing agency. Where the provisions of the above referenced documents are in conflict, the most restrictive requirement shall govern.</p> <p>3. Do not drill through existing embedded reinforcing steel. Locate existing reinforcing steel, create templates and adjust locations of holes in members being attached to clear existing embedded reinforcing.</p> <p>4. Do not exceed minimum or maximum bolt spacings or edge distance shown on the manufacturer's literature.</p> <p>5. All abandoned holes drilled in the concrete shall be completely filled with epoxy.</p> <p>6. Typically, holes in connection plates shall be no more than 1/16" larger than the adhesive anchor rod diameter. If larger diameter holes are used for erection purposes the contractor must provide plate washers. Plate washers must be welded to the connection plate to transfer the load. Welding must take place after holes are drilled, but prior to adhesive installation to avoid burning the adhesive.</p> <p>Progress Inspection Items</p> <p>1. The following Progress Inspection Items shall be required for construction. A qualified Special Inspection Agency shall be retained by the contractor to perform such inspections. A. Foundation and Footings B. Masonry C. Final</p>																																																																																																																																																																																																																																																														
<p>Abbreviations</p> <table border="0"> <tr><td>Alternate</td><td>ALT</td></tr> <tr><td>Anchor Bolt</td><td>AB</td></tr> <tr><td>Approximate</td><td>APPROX</td></tr> <tr><td>Architectural</td><td>ARCH</td></tr> <tr><td>Assembly</td><td>ASSY</td></tr> <tr><td>Average</td><td>AVG</td></tr> <tr><td>Back to Back</td><td>B TO B</td></tr> <tr><td>Base</td><td>BP</td></tr> <tr><td>Beam</td><td>BM</td></tr> <tr><td>Bending Moment</td><td>M</td></tr> <tr><td>Block</td><td>BLK</td></tr> <tr><td>Bottom</td><td>BOT or B</td></tr> <tr><td>Bottom Chord</td><td>BC</td></tr> <tr><td>Bottom of Steel</td><td>B/S</td></tr> <tr><td>Building</td><td>BLDG</td></tr> <tr><td>Cantilever</td><td>CANT</td></tr> <tr><td>Ceiling</td><td>CLG</td></tr> <tr><td>Center</td><td>CTR</td></tr> <tr><td>Centerline</td><td>CL</td></tr> <tr><td>Center to Center</td><td>C/C</td></tr> <tr><td>Clear</td><td>CLR</td></tr> <tr><td>Column</td><td>COL</td></tr> <tr><td>Concrete</td><td>CONC</td></tr> <tr><td>Connection</td><td>CONN</td></tr> <tr><td>Construction</td><td>CONST</td></tr> <tr><td>Construction Joint</td><td>CJ</td></tr> <tr><td>Continue / Continuous</td><td>CONT</td></tr> <tr><td>Contractor</td><td>CONTR</td></tr> <tr><td>Corporation</td><td>CORP</td></tr> <tr><td>Cover</td><td>CVR</td></tr> <tr><td>Cylinder</td><td>CYL</td></tr> <tr><td>Dead Load</td><td>DL</td></tr> <tr><td>Department</td><td>DEPT</td></tr> <tr><td>Detail</td><td>DET</td></tr> <tr><td>Diagonal</td><td>DIAG</td></tr> <tr><td>Diameter</td><td>DIA or Ø</td></tr> <tr><td>Direction</td><td>DIR</td></tr> <tr><td>Distance</td><td>DIST</td></tr> <tr><td>Dowel</td><td>DWL</td></tr> <tr><td>Down</td><td>DN</td></tr> <tr><td>Drawing</td><td>DRWG</td></tr> <tr><td>Each</td><td>EA</td></tr> <tr><td>Each Face</td><td>EF</td></tr> <tr><td>Each Way</td><td>EW</td></tr> <tr><td>East</td><td>E</td></tr> <tr><td>Electrical</td><td>ELEC</td></tr> <tr><td>Elevation</td><td>ELEV</td></tr> <tr><td>Elevator</td><td>ELEV</td></tr> <tr><td>Engineer</td><td>ENGR</td></tr> <tr><td>Equal</td><td>EQ</td></tr> <tr><td>Existing</td><td>EXIST</td></tr> <tr><td>Expansion Joint</td><td>JT</td></tr> <tr><td>Exterior</td><td>EXT</td></tr> <tr><td>Fabricate</td><td>FAB</td></tr> <tr><td>Face to Face</td><td>F TO F</td></tr> <tr><td>Far Side</td><td>FS</td></tr> <tr><td>Feet</td><td>FT</td></tr> <tr><td>Finish</td><td>FIN</td></tr> <tr><td>Flange</td><td>FLG</td></tr> <tr><td>Plate</td><td>PL</td></tr> <tr><td>Point</td><td>PT</td></tr> <tr><td>Pounds per square inch</td><td>PSI</td></tr> <tr><td>Prefabricated</td><td>PREFAB</td></tr> <tr><td>Prestressed</td><td>PST</td></tr> <tr><td>Project</td><td>PROJ</td></tr> <tr><td>Radius</td><td>R or RAD</td></tr> <tr><td>Reference</td><td>REF</td></tr> <tr><td>Reference Line</td><td>REF L</td></tr> <tr><td>Reinforce</td><td>REINF</td></tr> <tr><td>Required</td><td>REQD</td></tr> <tr><td>Right Hand</td><td>RH</td></tr> <tr><td>Roof Drain</td><td>RD</td></tr> <tr><td>Rough Opening</td><td>RO</td></tr> <tr><td>Schedule</td><td>SCH</td></tr> <tr><td>Section</td><td>SECT</td></tr> <tr><td>Separate</td><td>SEP</td></tr> <tr><td>Sheet</td><td>SH</td></tr> <tr><td>Short Leg Horizontal</td><td>SLH</td></tr> <tr><td>Short Leg Vertical</td><td>SLV</td></tr> <tr><td>Side</td><td>SD</td></tr> <tr><td>Similar</td><td>SIM</td></tr> <tr><td>Slope</td><td>SL</td></tr> <tr><td>Slotted</td><td>SLOT</td></tr> <tr><td>South</td><td>S</td></tr> <tr><td>Specification</td><td>SPEC</td></tr> <tr><td>Square</td><td>SQ</td></tr> <tr><td>Square Foot</td><td>SQ FT</td></tr> <tr><td>Square Inch</td><td>SQ IN</td></tr> <tr><td>Stainless Steel</td><td>SS</td></tr> <tr><td>Standard</td><td>STD</td></tr> <tr><td>Steel</td><td>STL</td></tr> <tr><td>Stiffener</td><td>STIFF</td></tr> <tr><td>Stone</td><td>STN</td></tr> <tr><td>Structural</td><td>STRUC</td></tr> <tr><td>Surface</td><td>SURF</td></tr> <tr><td>Symmetrical</td><td>SYM</td></tr> 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<tr><td>Yard</td><td>YD</td></tr> <tr><td>Direction Abbreviation</td><td>N-E</td></tr> </table>													Alternate	ALT	Anchor Bolt	AB	Approximate	APPROX	Architectural	ARCH	Assembly	ASSY	Average	AVG	Back to Back	B TO B	Base	BP	Beam	BM	Bending Moment	M	Block	BLK	Bottom	BOT or B	Bottom Chord	BC	Bottom of Steel	B/S	Building	BLDG	Cantilever	CANT	Ceiling	CLG	Center	CTR	Centerline	CL	Center to Center	C/C	Clear	CLR	Column	COL	Concrete	CONC	Connection	CONN	Construction	CONST	Construction Joint	CJ	Continue / Continuous	CONT	Contractor	CONTR	Corporation	CORP	Cover	CVR	Cylinder	CYL	Dead Load	DL	Department	DEPT	Detail	DET	Diagonal	DIAG	Diameter	DIA or Ø	Direction	DIR	Distance	DIST	Dowel	DWL	Down	DN	Drawing	DRWG	Each	EA	Each Face	EF	Each Way	EW	East	E	Electrical	ELEC	Elevation	ELEV	Elevator	ELEV	Engineer	ENGR	Equal	EQ	Existing	EXIST	Expansion Joint	JT	Exterior	EXT	Fabricate	FAB	Face to Face	F TO F	Far Side	FS	Feet	FT	Finish	FIN	Flange	FLG	Plate	PL	Point	PT	Pounds per square inch	PSI	Prefabricated	PREFAB	Prestressed	PST	Project	PROJ	Radius	R or RAD	Reference	REF	Reference Line	REF L	Reinforce	REINF	Required	REQD	Right Hand	RH	Roof Drain	RD	Rough Opening	RO	Schedule	SCH	Section	SECT	Separate	SEP	Sheet	SH	Short Leg Horizontal	SLH	Short Leg Vertical	SLV	Side	SD	Similar	SIM	Slope	SL	Slotted	SLOT	South	S	Specification	SPEC	Square	SQ	Square Foot	SQ FT	Square Inch	SQ IN	Stainless Steel	SS	Standard	STD	Steel	STL	Stiffener	STIFF	Stone	STN	Structural	STRUC	Surface	SURF	Symmetrical	SYM	System	SYS	Technical	TECH	Temperature	TEMP	Tension	TENS	Thick / Thickness	THK	Thread	THD	Tongue and Groove	T & G	Top and Bottom	T & B	Top of Concrete	T/C	Top of Masonry	T/M	Top of Steel	T/S	Top of Wood Framing	T/W	Total	TOT	Typical	TYP	Unless Otherwise Noted	U.O.N.	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P: (646) 912-8887 | e-mail: info@craftengin.com

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Job

Sheet

Structural Notes

Released For: Construction

Project #

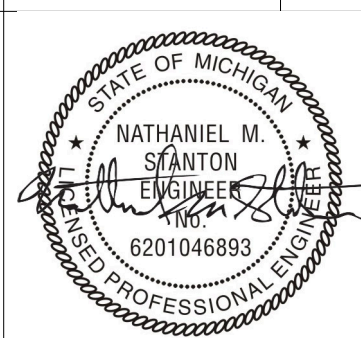
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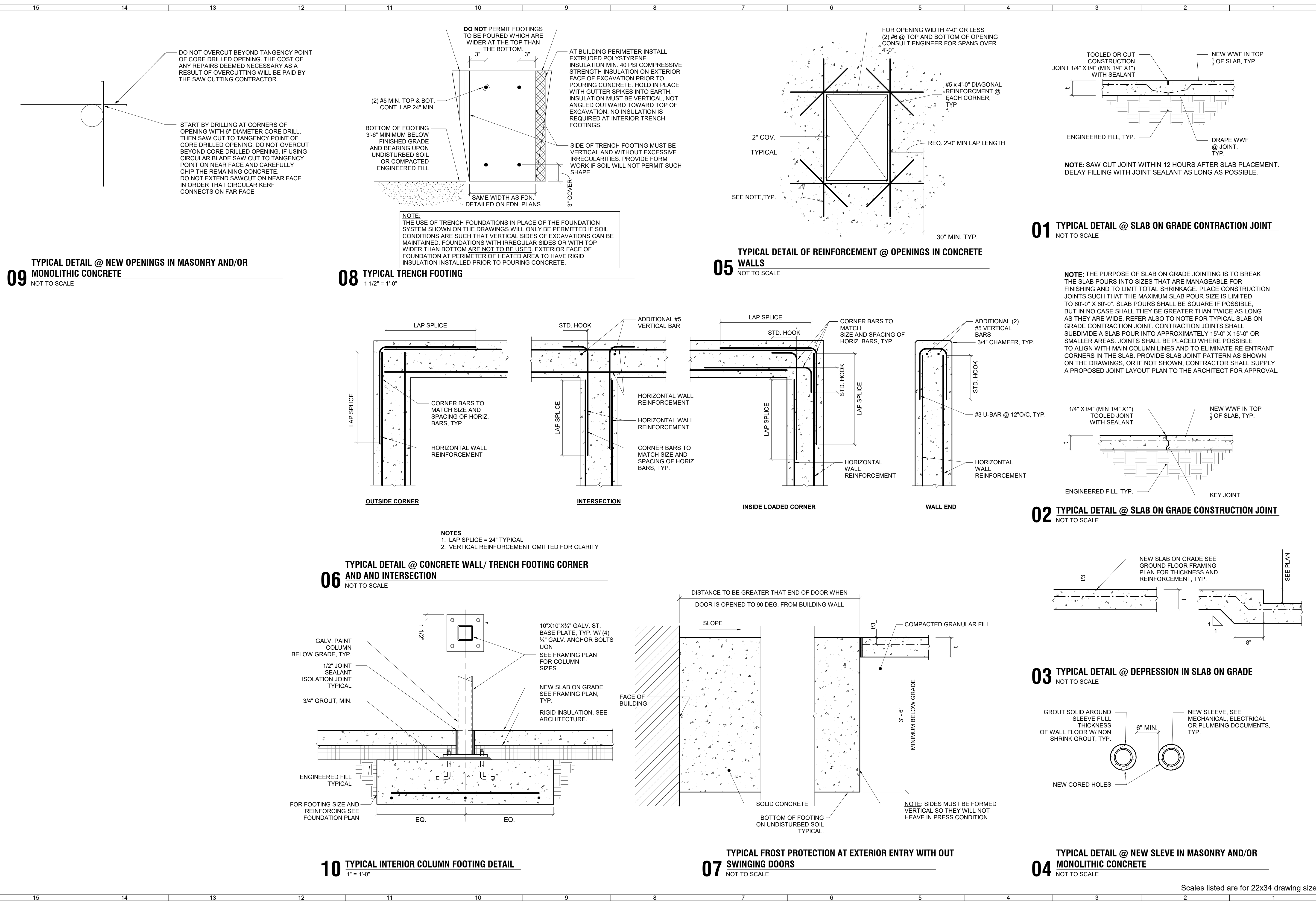
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Scales listed are for 22x34 drawing size

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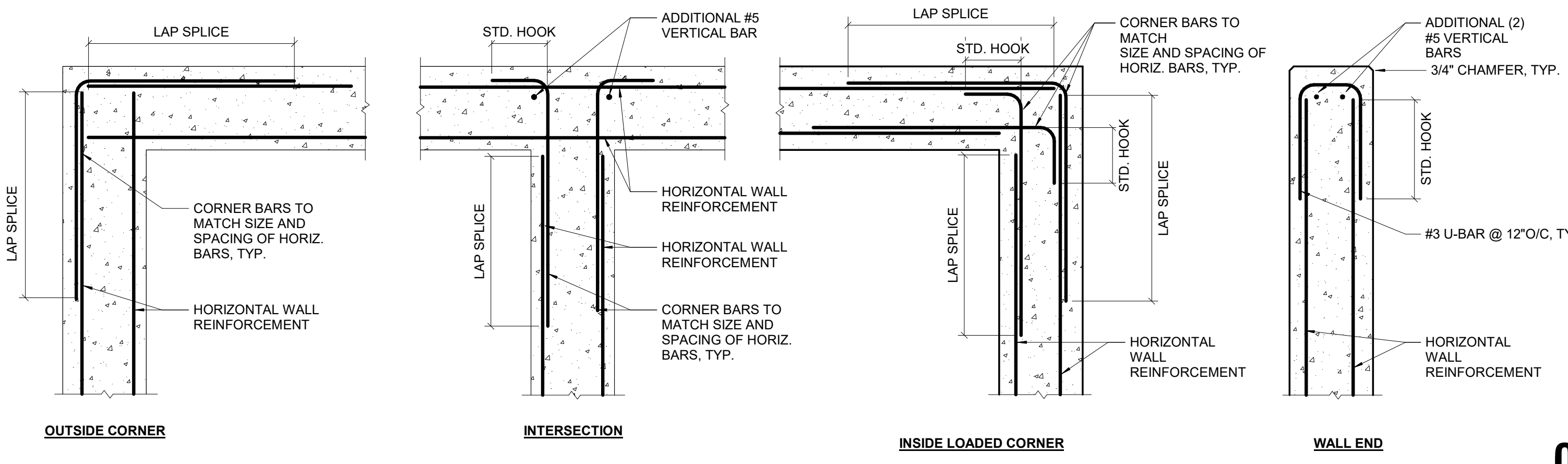


09 TYPICAL DETAIL @ NEW OPENINGS IN MASONRY AND/OR MONOLITHIC CONCRETE
NOT TO SCALE

08 TYPICAL TRENCH FOOTING
1 1/2" = 1'-0"

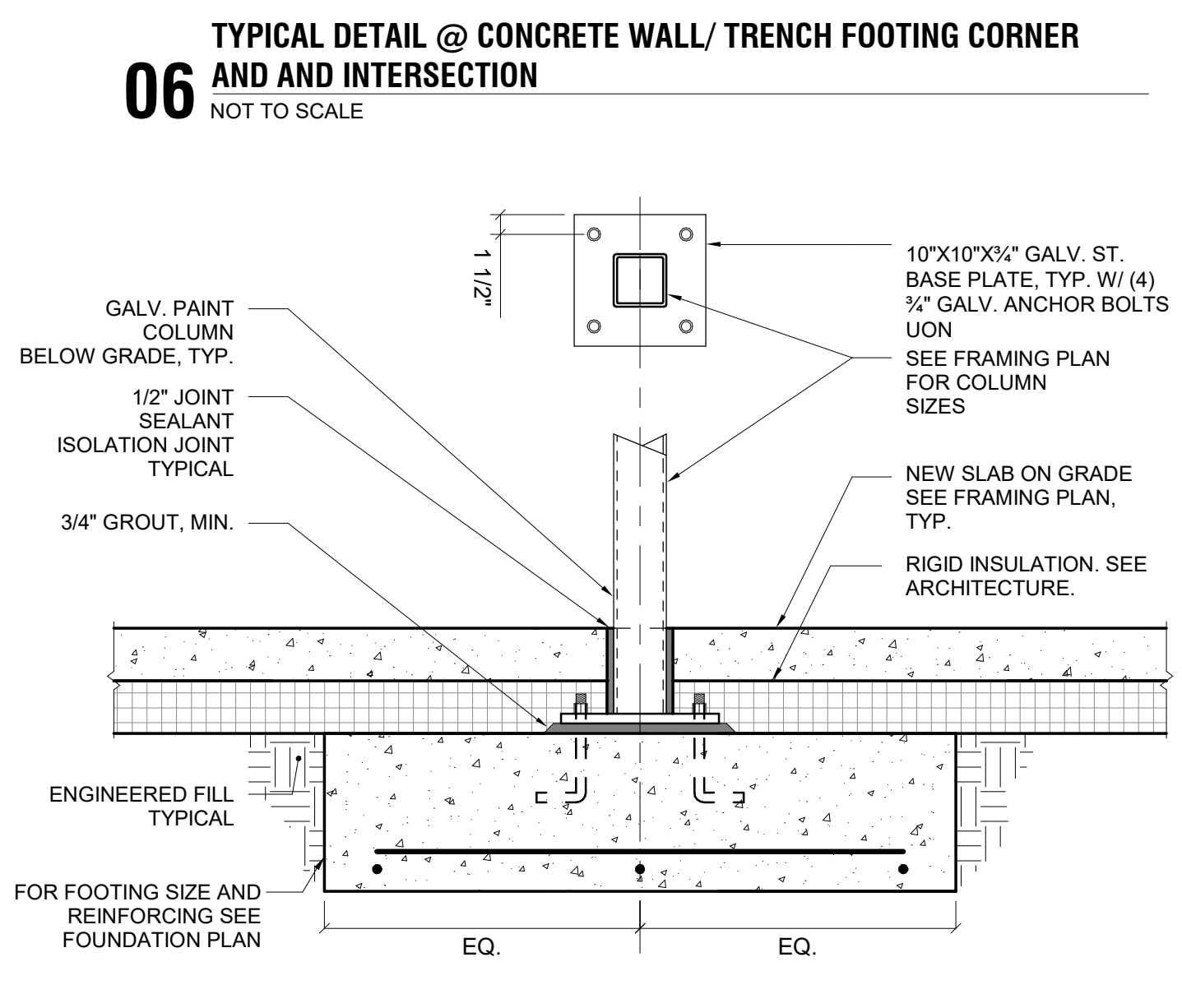
05 TYPICAL DETAIL OF REINFORCEMENT @ OPENINGS IN CONCRETE WALLS
NOT TO SCALE

01 TYPICAL DETAIL @ SLAB ON GRADE CONTRACTION JOINT
NOT TO SCALE

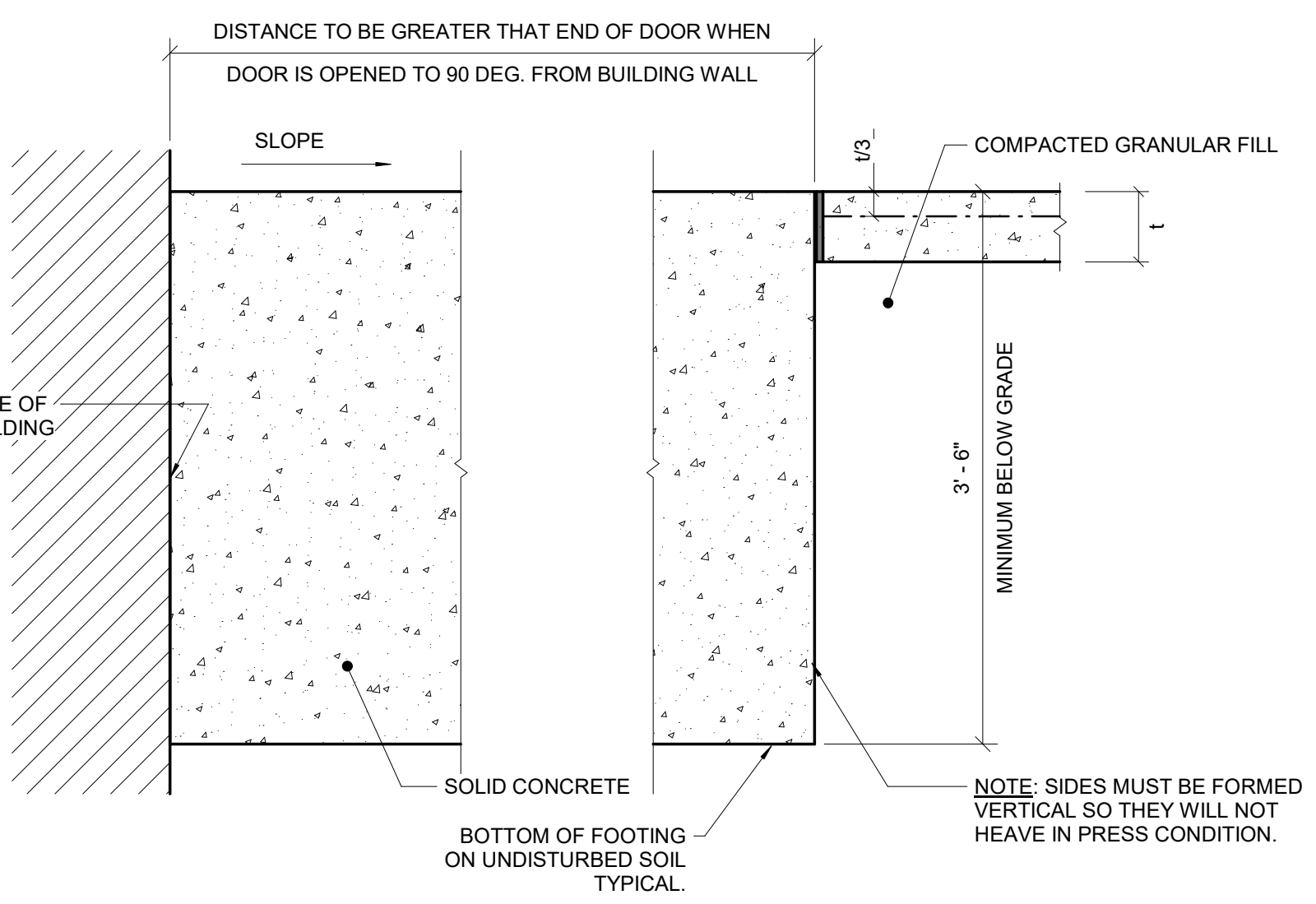


06 TYPICAL DETAIL @ CONCRETE WALL/ TRENCH FOOTING CORNER AND AND INTERSECTION
NOT TO SCALE

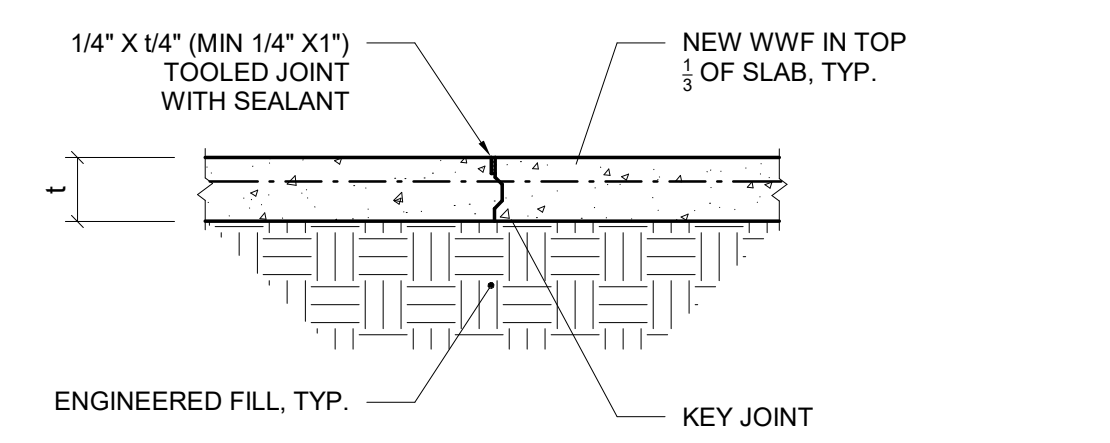
02 TYPICAL DETAIL @ SLAB ON GRADE CONSTRUCTION JOINT
NOT TO SCALE



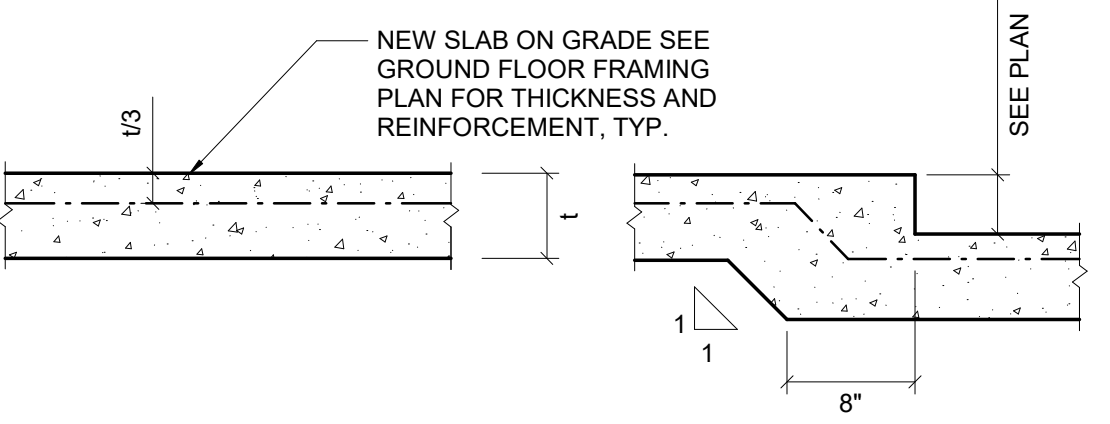
10 TYPICAL INTERIOR COLUMN FOOTING DETAIL
1" = 1'-0"



07 TYPICAL FROST PROTECTION AT EXTERIOR ENTRY WITH OUT SWINGING DOORS
NOT TO SCALE

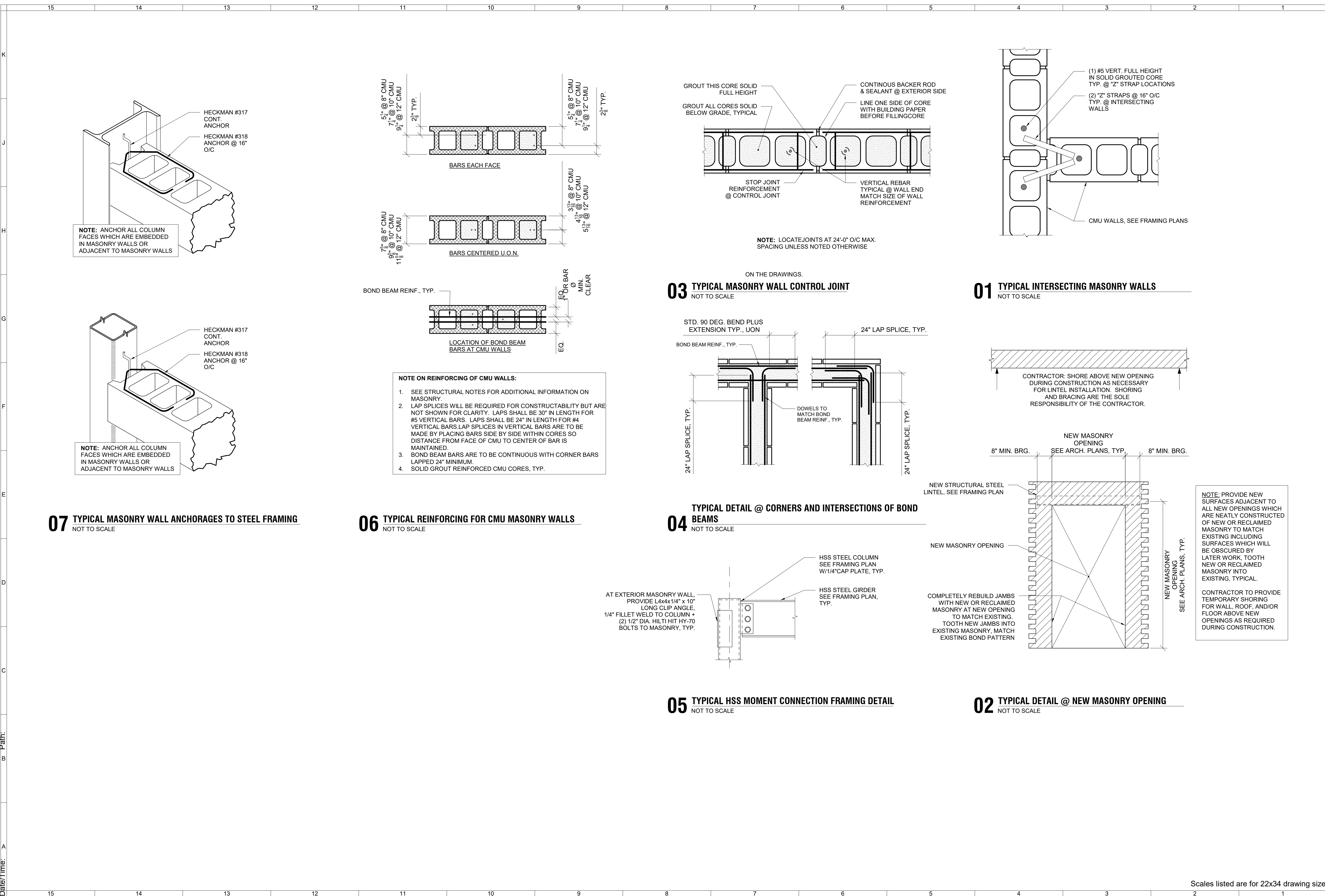


03 TYPICAL DETAIL @ DEPRESSION IN SLAB ON GRADE
NOT TO SCALE

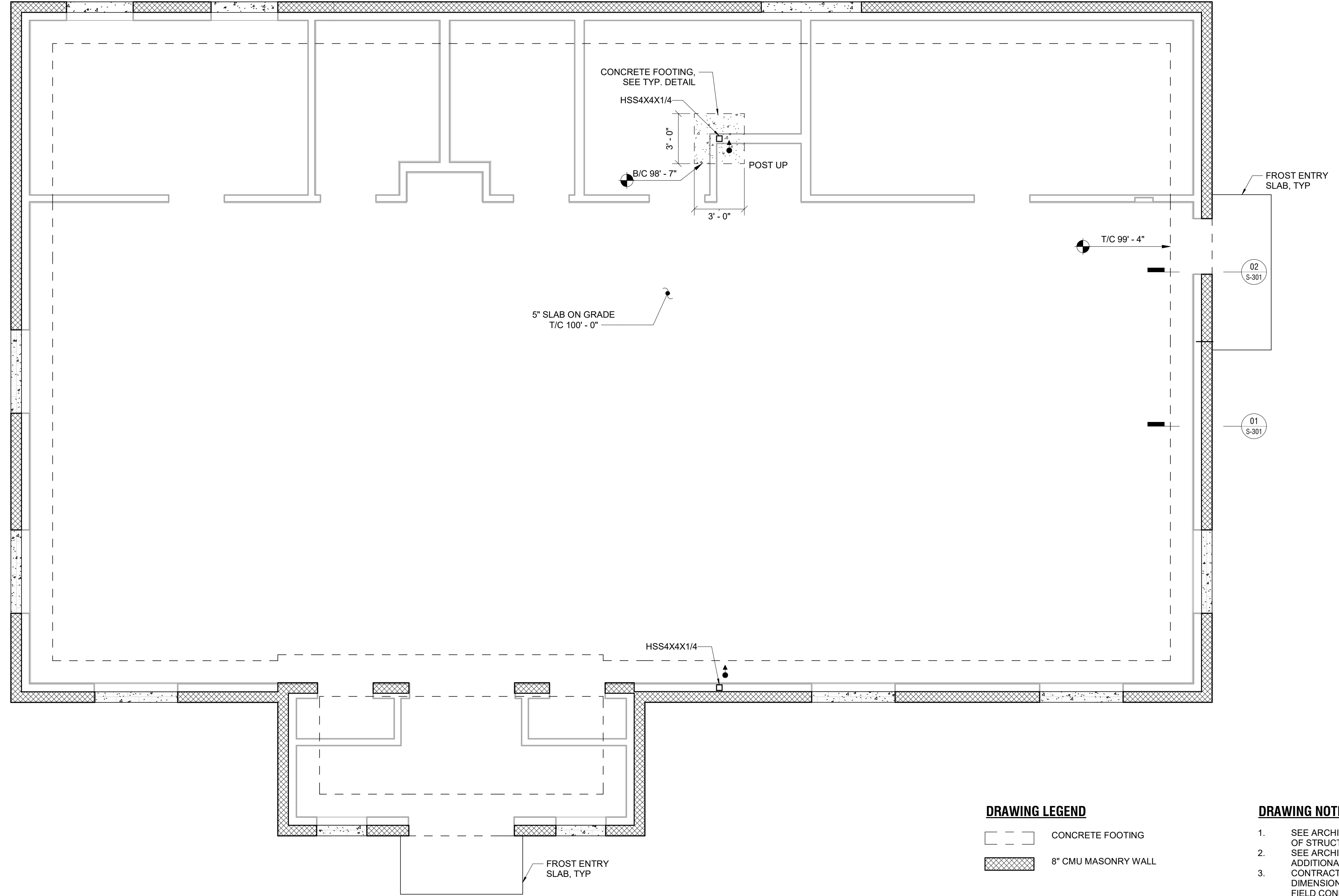


04 TYPICAL DETAIL @ NEW SLEEVE IN MASONRY AND/OR MONOLITHIC CONCRETE
NOT TO SCALE

Plot Date/Time: 3/22/2018 10:56:53 AM File Path: C:\Users\etmaglar\CRAFTNET\Documents\180227 - Dexter Lab_etmaglar.rvt



Scales listed are for 22x34 drawing size



01 Foundation Framing Plan
1/4" = 1'-0"

DRAWING LEGEND

- CONCRETE FOOTING
- 8" CMU MASONRY WALL

DRAWING NOTES

1. SEE ARCHITECT FOR FIREPROOFING REQUIREMENTS OF STRUCTURAL ITEMS
2. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS
3. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND FIELD CONDITIONS. REPORT ANY DISCREPANCIES IMMEDIATELY TO ARCHITECT.

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39 West 38th Street, Suite 6E
New York, NY 10018
P: (646) 912-8867 | e-mail: info@craftengin.com

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Dexter, MI 48130

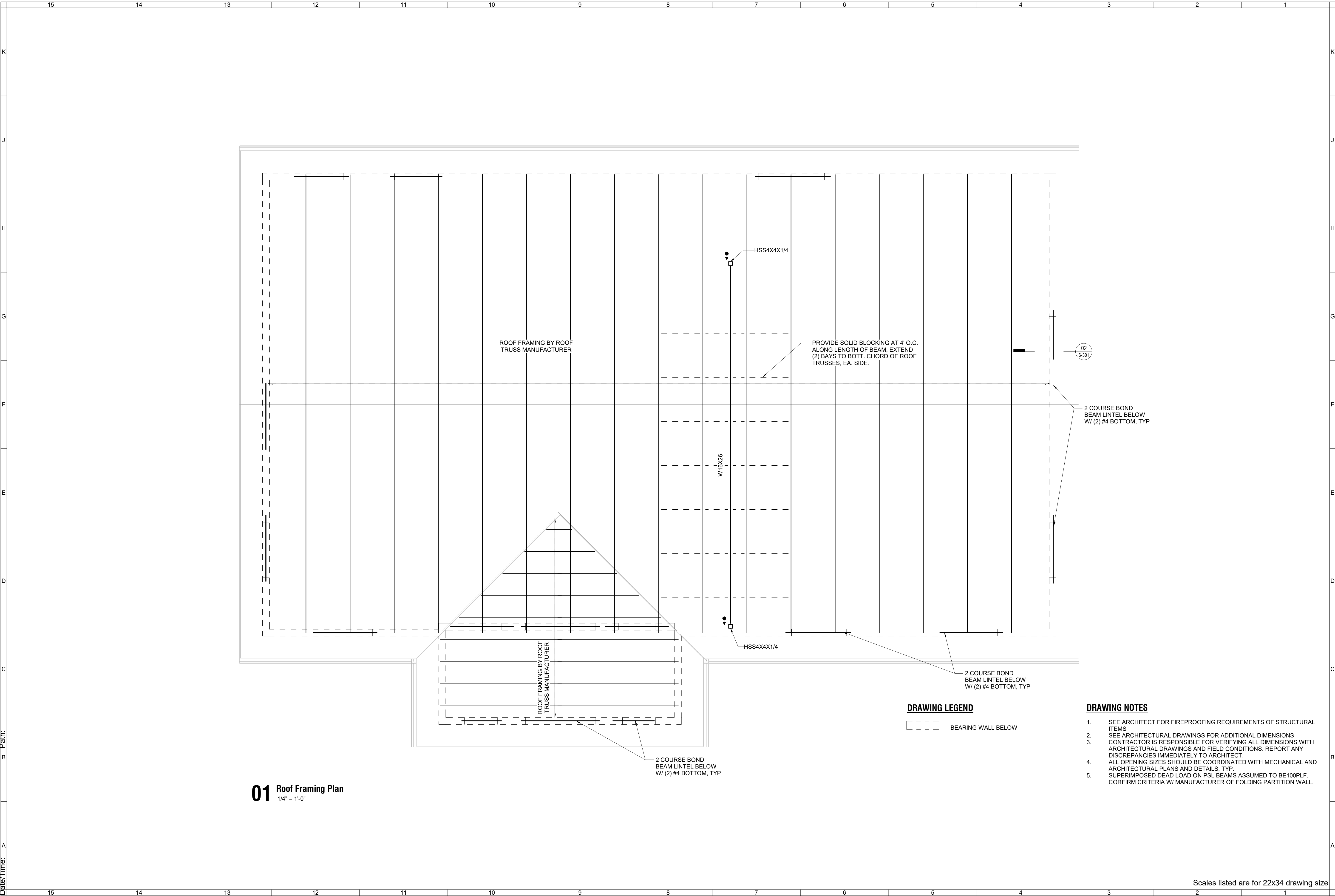
Sheet
Foundation Framing Plan
Released For: Construction



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Project # | Dwg Date
Drawing #



01 Roof Framing Plan
1/4" = 1'-0"

DRAWING LEGEND

--- BEARING WALL BELOW

DRAWING NOTES

1. SEE ARCHITECT FOR FIREPROOFING REQUIREMENTS OF STRUCTURAL ITEMS
2. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONS
3. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND FIELD CONDITIONS. REPORT ANY DISCREPANCIES IMMEDIATELY TO ARCHITECT.
4. ALL OPENING SIZES SHOULD BE COORDINATED WITH MECHANICAL AND ARCHITECTURAL PLANS AND DETAILS, TYP.
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Scales listed are for 22x34 drawing size



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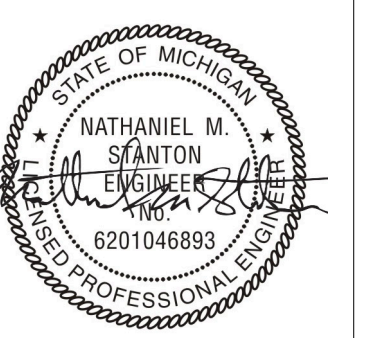
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New York, NY 10018
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Sheet
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Released For: Construction



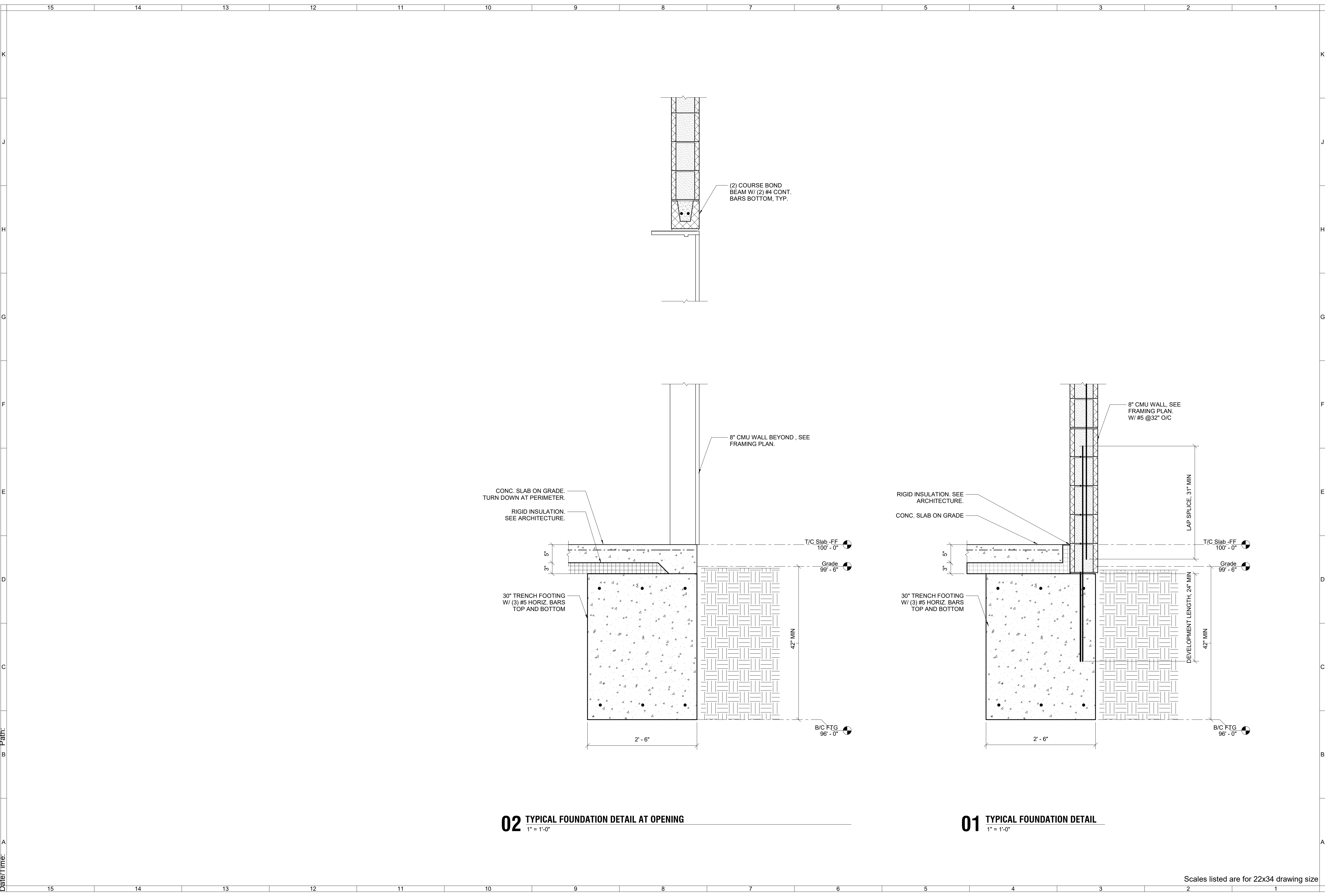
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Project # | Dwg Date
Drawing #




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02 TYPICAL FOUNDATION DETAIL AT OPENING
1" = 1'-0"

01 TYPICAL FOUNDATION DETAIL
1" = 1'-0"

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Job	Dexter Alternative Education Building 8100 Shield Rd Dexter, MI 48130
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Date	03/21/2018
Project #	17054
Drawing #	S-301